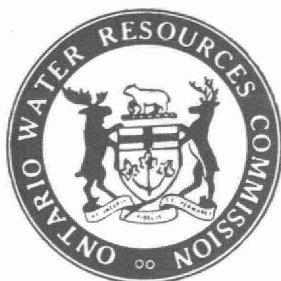


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*Research paper 2007*

HYDRAULIC CHARACTERISTICS  
OF  
COMMON WATER FILTER MEDIA

DIVISION OF RESEARCH  
ONTARIO WATER RESOURCES COMMISSION



October 1968

R.P. 2007

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HYDRAULIC CHARACTERISTICS  
OF  
COMMON WATER FILTER MEDIA

By  
M. B. Fielding  
October 1968

Division of Research  
Paper No. 2007

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The Ontario Water Resources Commission

## INTRODUCTION

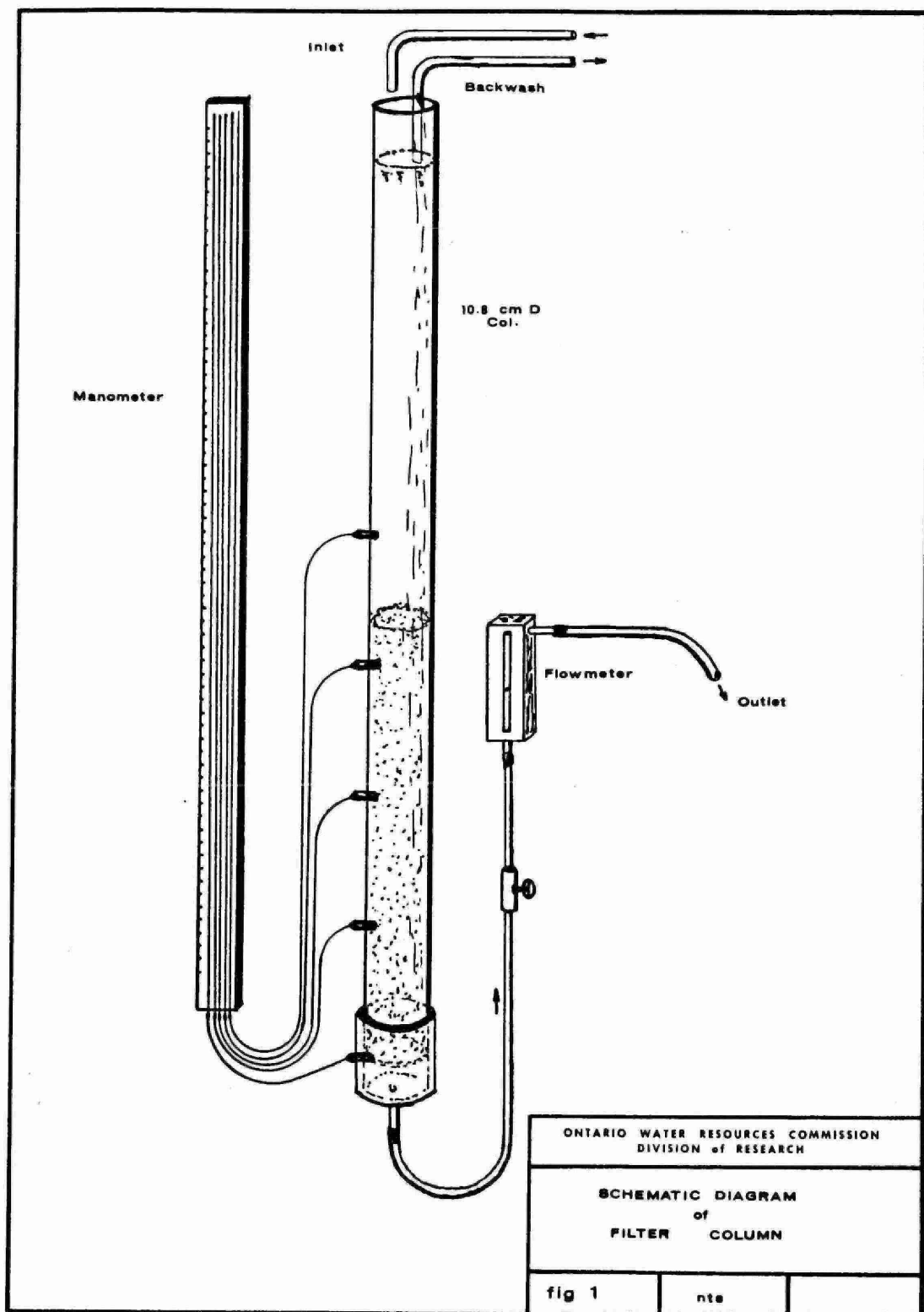
This paper is designed to provide basic hydraulic data pertaining to filter media commonly used in water treatment filters. For this purpose the following materials were selected for evaluation:

- 1) "Ottawa" sand
- 2) Anthracite coal - "Anthrafilt" #1-1/2
- 3) Anthracite coal - "Anthrafilt" #2

The data obtained are applicable to the design of both conventional rapid sand filters and multi-media filters in that initial and relative head-loss may be determined.

## EQUIPMENT AND PROCEDURE

A filter column as shown in Figure 1 was constructed in the laboratory. The column consisted of an acrylic tube, 155 cm long with a 10.8 cm inside diameter, open at the top and closed with a gasketted, slide-fit cap at the bottom. The cap, with central 13 mm pipe outlet, supported a layer of 15 mm diameter glass beads upon which a 595 micron screen was supported. The filter media was placed directly on this screen. Manometer taps were inserted into the column immediately above the screen (to eliminate effects of system head loss) and at 20 cm vertical intervals above the screen. These taps were connected to a multi-bank manometer. Influent, consisting of tap water, was provided through rubber tubing connected to the laboratory potable water supply with free discharge to the top of the filter column. Effluent was discharged through the bottom pipe and flexible tubing to a flow meter (Fisher and Porter 1027 "Flowrator") and from there to waste. A control valve on the discharge line regulated the flow. Backwash was accomplished by connecting the discharge pipe to the potable water supply with backwash wastewater discharged through a syphon (with aspirator assist) at the top of the column. The entire assembly was supported on a free-standing angle iron frame.



The test procedure used for each type of media was similar. An amount of raw media was added to the column to give a workable filter depth and backwashed at a rate, and for a time, adequate to remove all dirt particles and extremely fine material. The backwash was shut off, the media allowed to settle, and the feed started. After the bed had stabilized the depth was measured. Flow rate was varied over the test range and bed head-loss measured at each flow rate increment. Each test run was duplicated by starting at a low flow rate, increasing in increments to the maximum rate and then decreasing by the same increments back to the starting rate. Throughout the test the influent water temperature was maintained at  $10^{\circ}\text{C} \pm 1^{\circ}$ . After each test the filter was backwashed and the procedure repeated. At the conclusion of a test series the media was recovered, air-dried to a surface dry condition and weighed. A representative sample was submitted to a sieve analysis and the porosity of the material obtained by the displacement principle. (Sample calculations are included in Appendix A)

### TEST RESULTS

The results of the sieve analyses for the three media tested are shown in Figure 2. Analyses were done using the International (ISO) Standard sieve series plus intermediate sieves to provide additional values where required. Table 1 presents a summary of these results.

TABLE 1

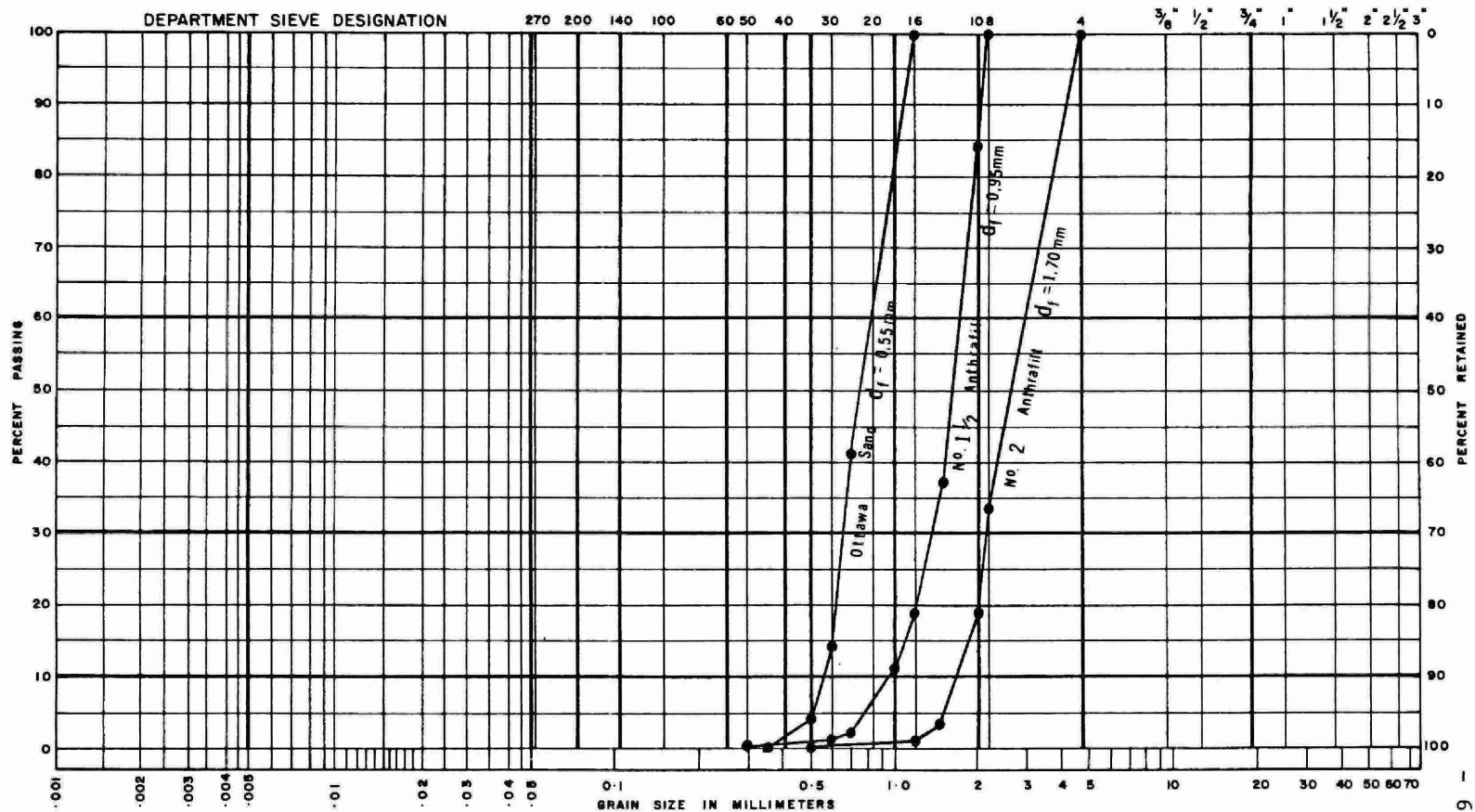
	<u>df</u>	<u>UC</u>
Ottawa Sand	0.55 mm	1.49
Anthrafilt #1-1/2	0.95 mm	1.84
Anthrafilt #2	1.70 mm	1.77

Results of the head-loss tests for the three filter media are shown in Figure 3. It will be noted that more than one value of porosity (f)\* is shown for each filter material. The values shown are those obtained when the material settled after backwash and represent the variations in porosity which may occur in actual practice. The effects of the variation in f are significant at high flow rates.

\*porosity - f - is defined as the ratio of the void volume to the total volume of material including solids and voids.



U. S. BUREAU OF SOILS CLASSIFICATION							
Clay	Silt	Very Fine Sand	Fine Sand	Medium Sand	Coarse Sand	Fine Gravel	Gravel



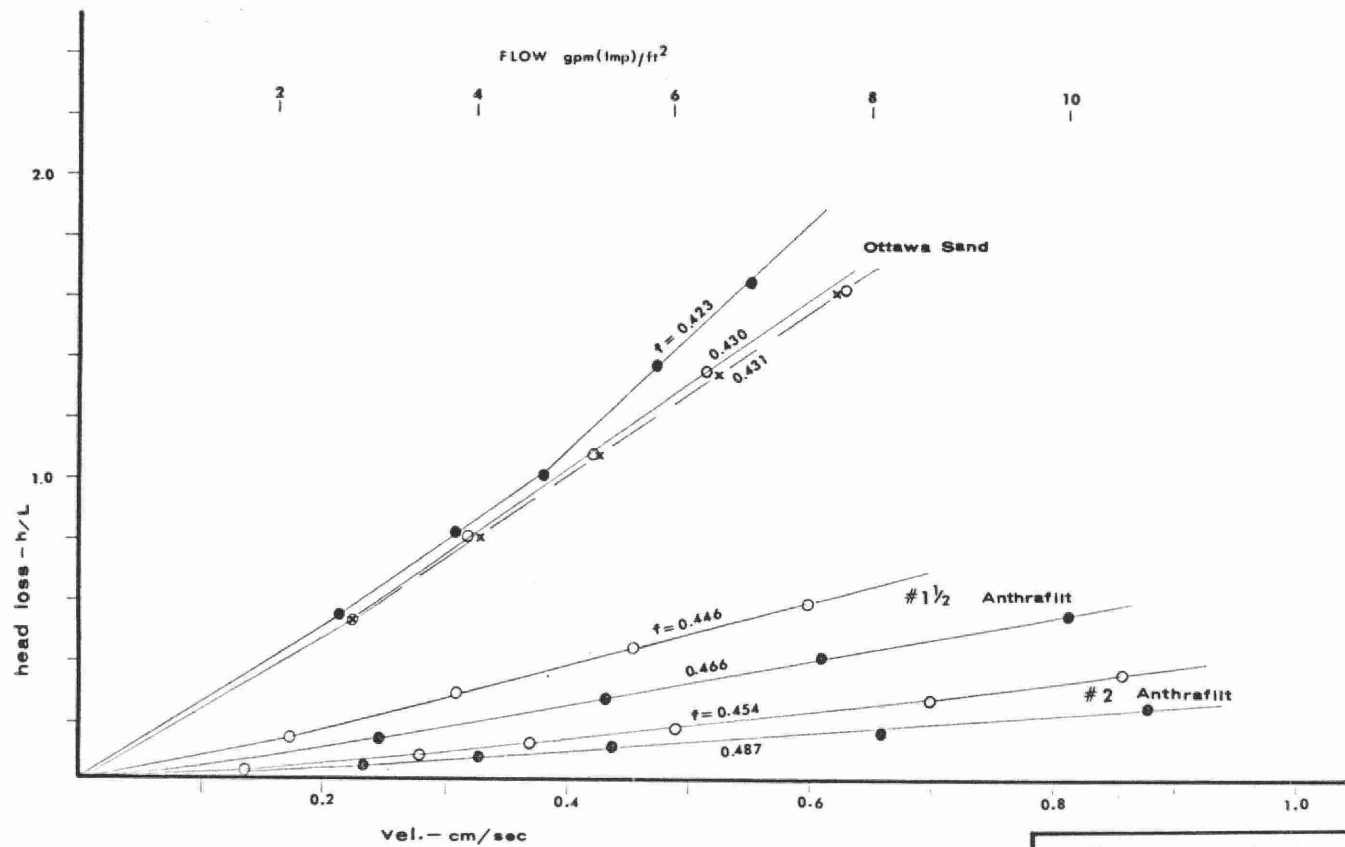
Clay & Silt	Sand			Gravel	
	Fine	Medium	Coarse	Fine	Coarse
UNIFIED SOIL CLASSIFICATION SYSTEM					

REMARKS FILTER MEDIA SIEVE ANALYSES

- Samples graded after backwashing

DATE March 1967

Fig. 2



$t = 10^{\circ}C$

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HEAD LOSS vs VELOCITY

fig 3

# CONCLUSION

The mean experimental data for each of the filter media are plotted in Figure 4, along with the theoretical values as computed from the formula: (1)

$$h/L = 1.067 \frac{C_d}{g} \cdot \frac{1}{f^4} \cdot \frac{v^2}{d}$$

$$\text{where } C_d = \frac{24}{R} + \frac{3}{\sqrt{R}} + 0.34$$

$$R = \frac{vd}{\nu}$$

$$h/L = \text{head loss per unit filter depth}$$

$$f = \text{porosity}$$

$$v = \text{flow rate, cm/sec}$$

$$d = \text{effective grain size}$$

It will be noted that the experimental data are consistently below the theoretical values. This obtains in spite of plotting theoretical values for a media with sphericity ( $\psi$ ) of 1, i.e.: for spheres.

If it is assumed that flow through a filter is laminar and that the porosity ( $f$ ) will be approximately 0.4, a simplified calculation of head-loss is possible. These assumptions are generally valid as indicated by the porosity values obtained in the experimental runs, and by calculation of maximum values of Reynold's number ( $R$ ), at

a velocity of 1.0 cm/sec, grain size of 0.1 cm, and kinematic viscosity of 1.3 centistokes (T=10°C). At this point R = 7.7 which is still within the range of laminar flow. (2)

With the assumption of laminar flow

$$C_d = \frac{24}{R}$$

$$\text{where } R = \frac{vd}{\nu}$$

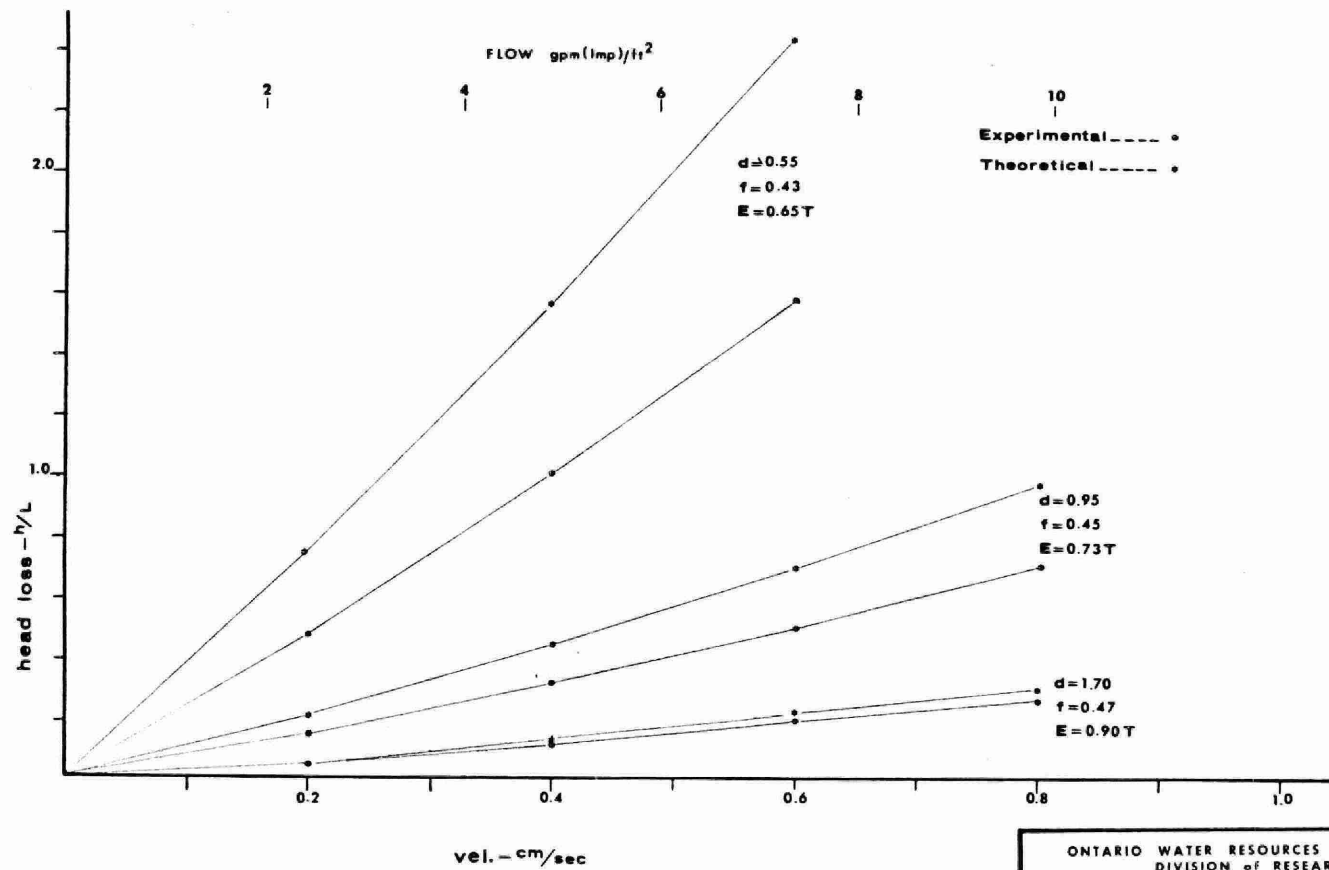
$$\text{and } h/L = 1.067 \frac{C_d}{g} \cdot \frac{1}{f^4} \cdot \frac{v^2}{d}$$

$$= 25.6 \nu/g \cdot \frac{1}{f^4} \cdot \frac{v}{d^2}$$

and if  $f = 0.4$

$$h/L = 1.02 \nu v/d^2 \quad (\text{metric units})$$

As previously stated the experimental results obtained were consistently lower than the theoretical values calculated by the above methods. This would indicate that a calculation of initial head-loss based on either of these methods will produce a conservative design. If initial head-loss is a critical factor in the design it is recommended that a model study be carried out.



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EXPERIMENTAL vs. THEORETICAL  
RESULTS

fig. 4

## APPENDIX A

# SAMPLE CALCULATIONS

Ottawa Sand

net weight of total sample (air dried) 9929.6 gms

## voids:

dry wt.	1,000 gms
loose vol. of sand	620 cm <sup>3</sup>
water added	<u>500</u> cm <sup>3</sup>
total vol.	1,120 cm <sup>3</sup>
apparent vol.	<u>880</u> cm <sup>3</sup>
therefore: void vol.	240 cm <sup>3</sup>

## porosity:

$$f = \frac{240}{620} = 0.387$$

when volume = 620 ml/1,000 gms

f = 0.387 when total sample occupies  
9.9296 x 620 = 6156.35 cm<sup>3</sup>

$$\text{Filter column} = \frac{\pi \times 10.76^2 \times 1}{4} = 90.9091 \text{ cm}^3/\text{cm}$$

sand depth = 73.5 cms (T101)

$$\text{therefore vol.} = 73.5 \times 90.9091 = 6681.9 \text{ cm}^3$$

$$f = \frac{6681.9}{6156.4} \times 0.387 = 0.420$$

## APPENDIX B



The following graphs giving theoretical unit head-loss values for various effective diameter particles at various flow rates were prepared from computer results of the empirical formula:

$$h/L = 1.067 \frac{C_d}{g} \cdot \frac{1}{f^4} \cdot \frac{v^2}{d}$$

$$\text{in which } C_d = \frac{24}{R} + \frac{3}{\sqrt{R}} + 0.34$$

$$g = 980.6 \text{ cm/sec/sec}$$

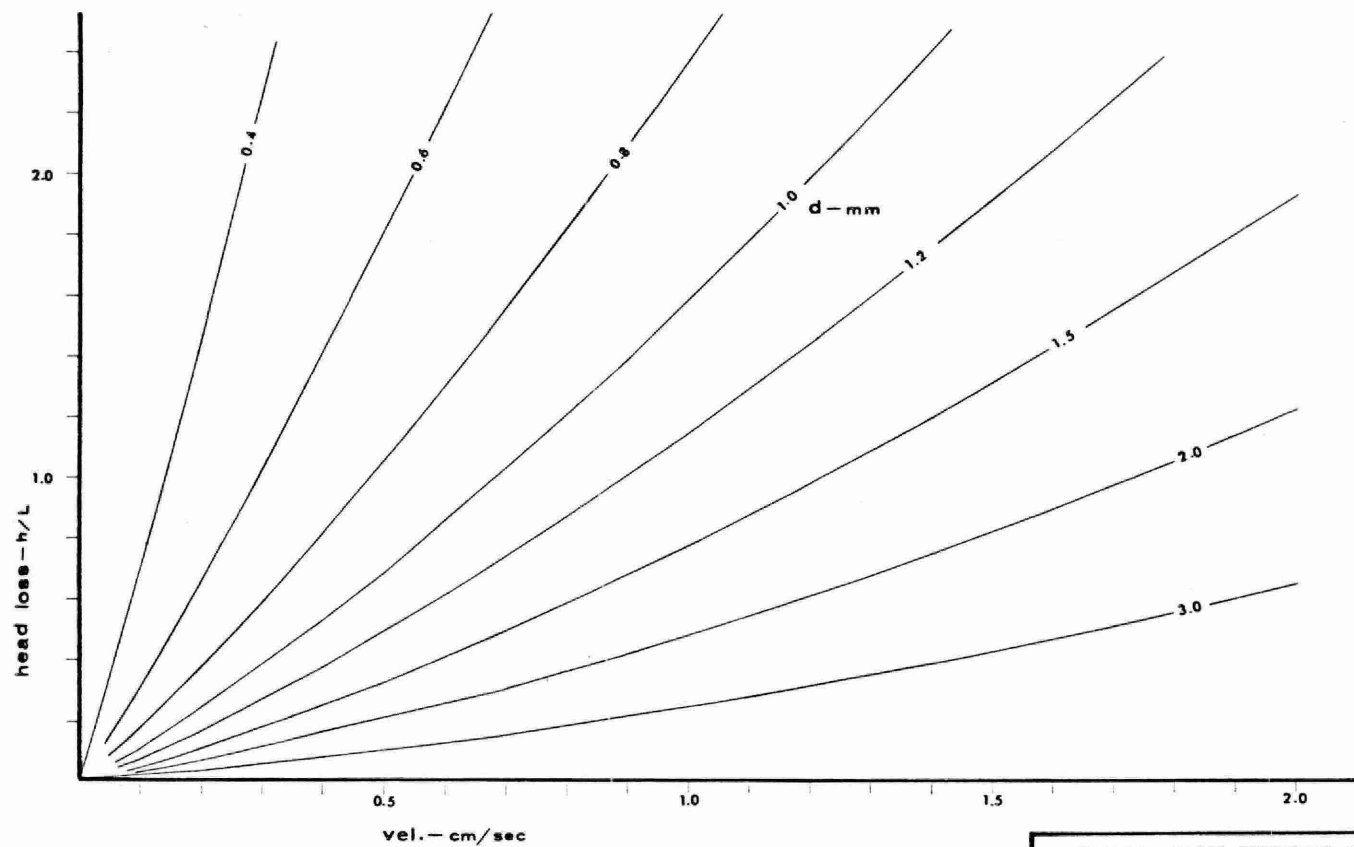
$$f = 0.400$$

$$v = \text{cm/sec}$$

$$d = \text{cm}$$

TABLE B1  
Porosity (f) Correction

f	Multiply $h/L$ by	f	Multiply $h/L$ by
0.30	3.160	0.45	0.624
0.31	2.783	0.46	0.571
0.32	2.440	0.47	0.525
0.33	2.159	0.48	0.482
0.34	1.916	0.49	0.444
0.35	1.707	0.50	0.410
0.36	1.524	0.51	0.378
0.37	1.366	0.52	0.350
0.38	1.228	0.53	0.324
0.39	1.107	0.54	0.301
0.40	1.000	0.55	0.280
0.41	0.906	0.56	0.260
0.42	0.823	0.57	0.242
0.43	0.749	0.58	0.226
0.44	0.683	0.59	0.211



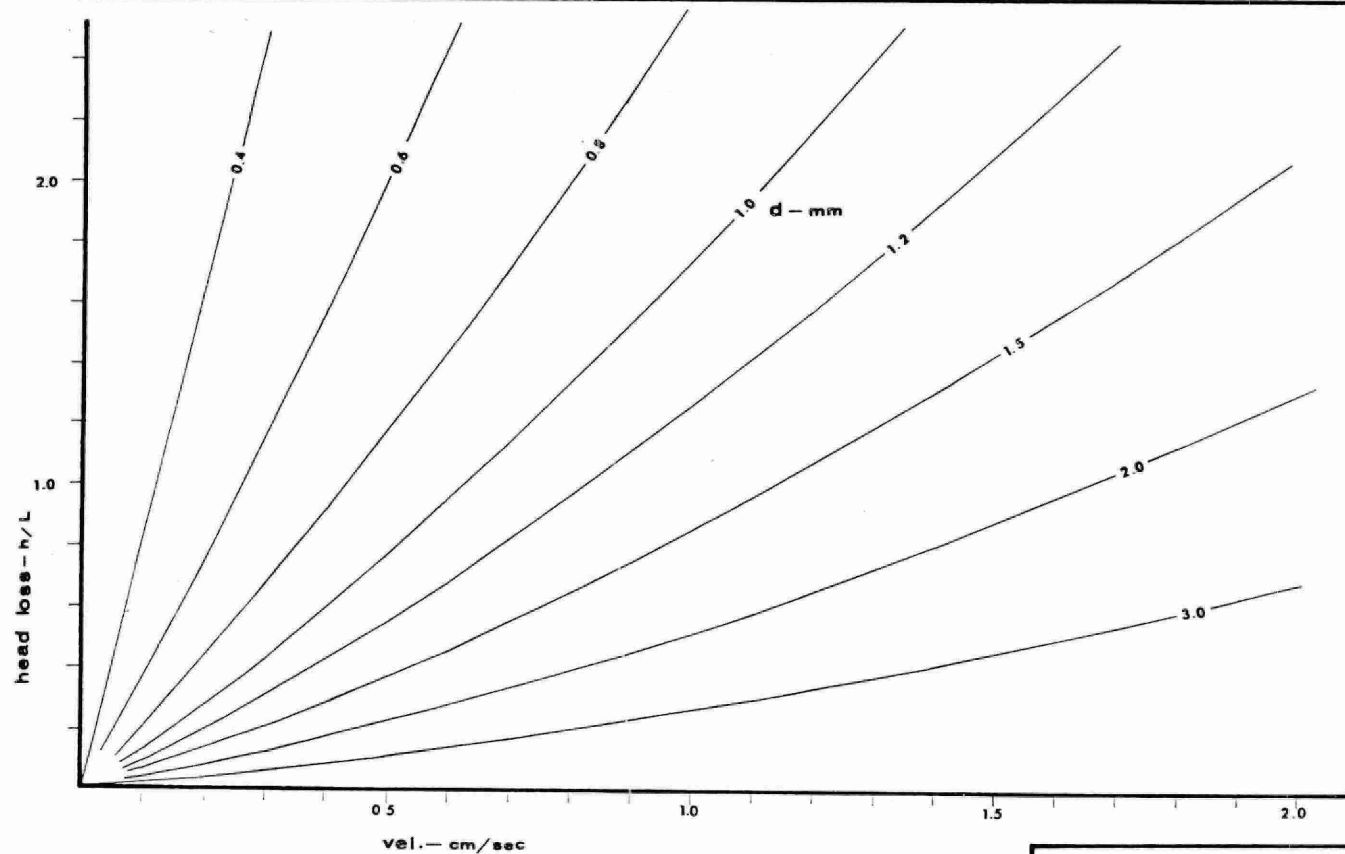
$t=20^{\circ}\text{C}$      $f=0.40$

$2 \text{ gpm (Imp)}/ft^2 = 0.163 \text{ cm/sec}$

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HEAD LOSS vs VELOCITY

fig 5



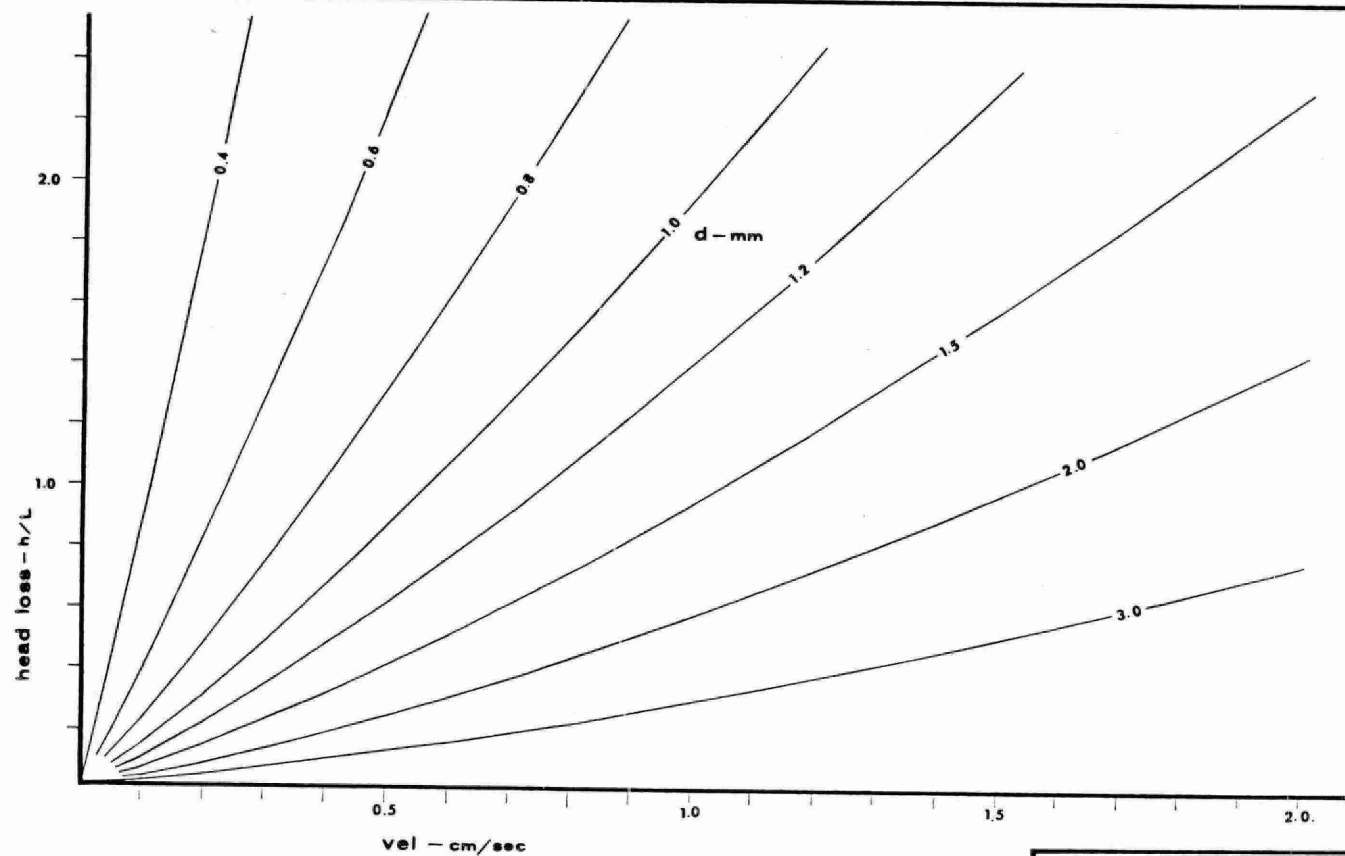
$t=15^{\circ}C$   $f=0.40$

$2 \text{ gpm(lmp)}/ft^2 = 0.163 \text{ cm/sec}$

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HEAD LOSS vs VELOCITY

fig 6



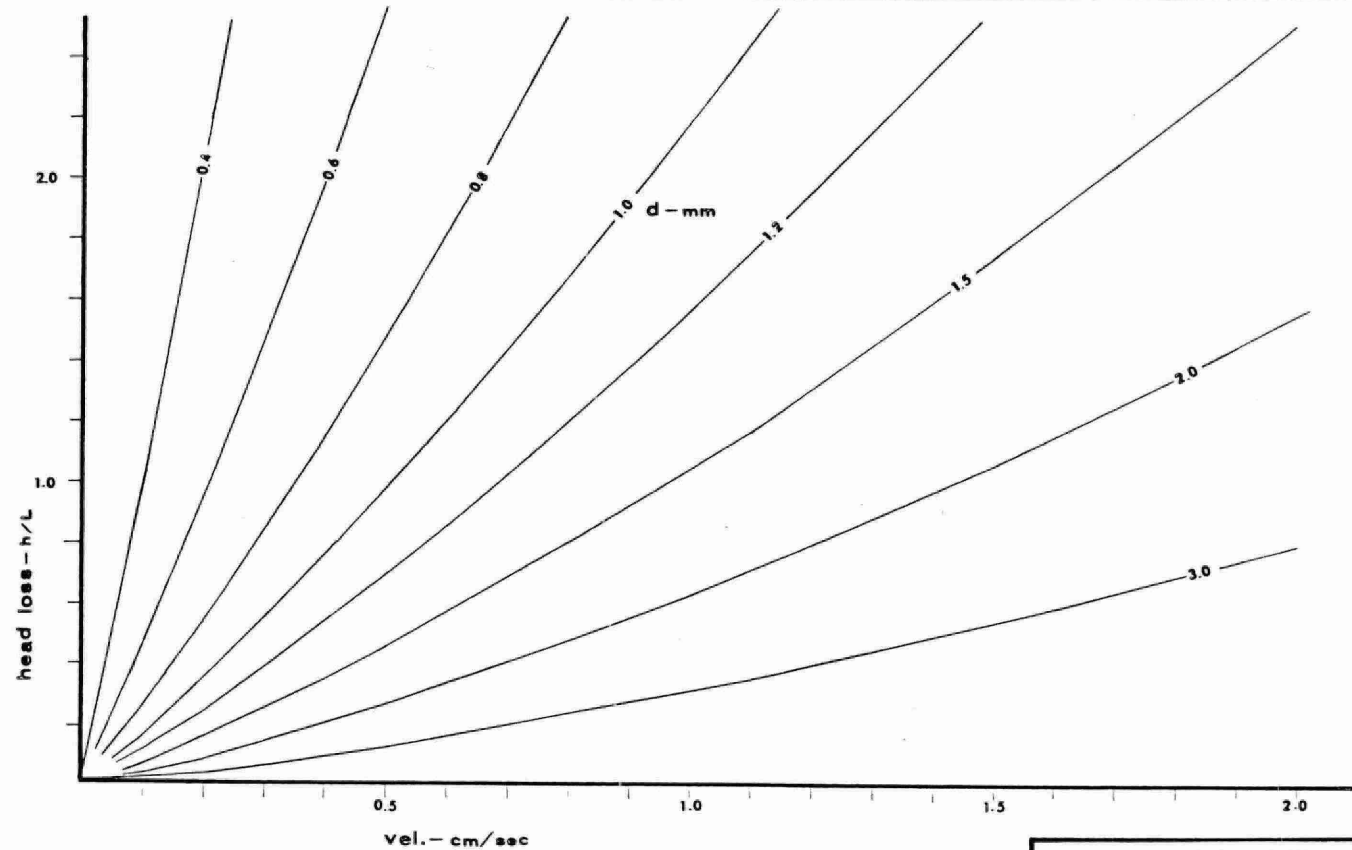
$t=10^0c$   $f=0.40$

$2 \text{ gpm (imp)}/ft^2 = 0.163 \text{ cm/sec}$

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HEAD LOSS vs VELOCITY

fig 7



$t = 5^{\circ}\text{C}$      $f = 0.40$

$2 \text{ gpm (imp) / ft}^2 = 0.163 \text{ cm/sec}$

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HEAD LOSS vs VELOCITY

fig 8

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